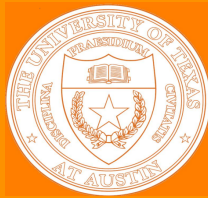


# *Characterizing Nonlinear Viscoelastic Response of Asphaltic Materials*



University of Texas at Austin


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## **Background**

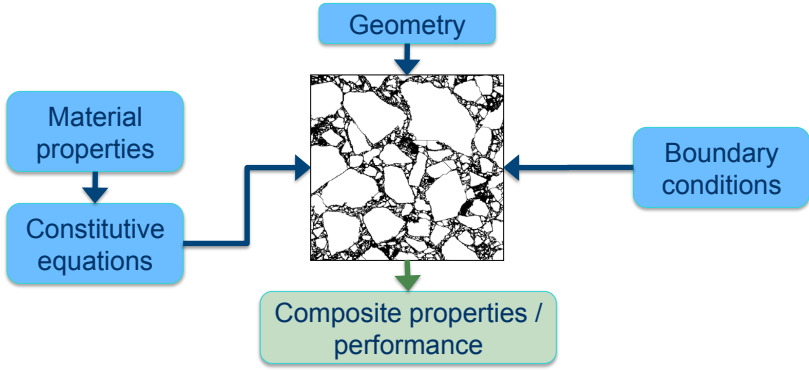
Computational models of asphalt composites are increasingly being used to:

- investigate relationship between constituent and mixture properties
- predict damage evolution in composites
- optimize mix design using virtual testing


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## Background


While these micromechanical computational models may vary in **length scale** (e.g. mastic, mortar, or mixture) or **technique** (e.g. FEM or DEM), there are some elements that are common to most of these models



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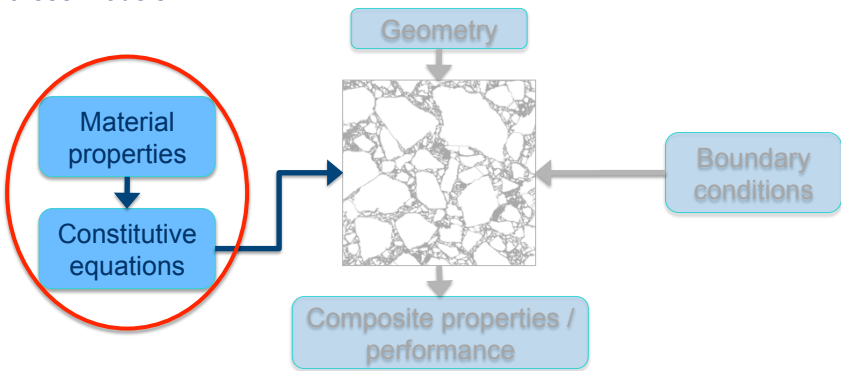
    graph TD
      Geometry[Geometry] --> Model[Microstructure Model]
      Material[Material properties] --> Equations[Constitutive equations]
      Equations --> Model
      BC[Boundary conditions] --> Model
      Model --> Output[Composite properties / performance]
  
```

The diagram illustrates the inputs and outputs of a micromechanical computational model. It features a central square representing a microstructure model, which is a network of interconnected nodes and lines. Four blue boxes are connected to this central model: 'Geometry' at the top, 'Material properties' and 'Constitutive equations' on the left, and 'Boundary conditions' on the right. Arrows point from each of these boxes towards the central model. Below the model, a green box labeled 'Composite properties / performance' has an arrow pointing downwards from the model to it.


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```

This diagram is identical to the one above, showing the flow from inputs to a central microstructure model and then to composite properties. However, in this version, the 'Material properties' and 'Constitutive equations' boxes, along with the arrow between them, are enclosed in a red circle to highlight their importance.

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## Objectives

Creep-recovery / Time sweep / Amplitude sweep

Obtain model parameters

- Power Law
- Prony Series

Input to computational model

Performance prediction

Material characterization

*Typical approach to characterize binder / matrix*

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## Objectives

Investigate linear and non-linear viscoelastic behavior of binders in typical torsion shear tests with emphasis on:

- sources of non-linear viscoelastic response
- constitutive equations that can be used to model this response

## Tests

- Creep and Recovery at different stress levels
- Cyclic Loading (Stress Amplitude Sweep)
- Cyclic Loading (Time Sweep at Different Stress Amplitudes)

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## Test

### Materials

- PG 82-22
- PG 76-22

### Tests

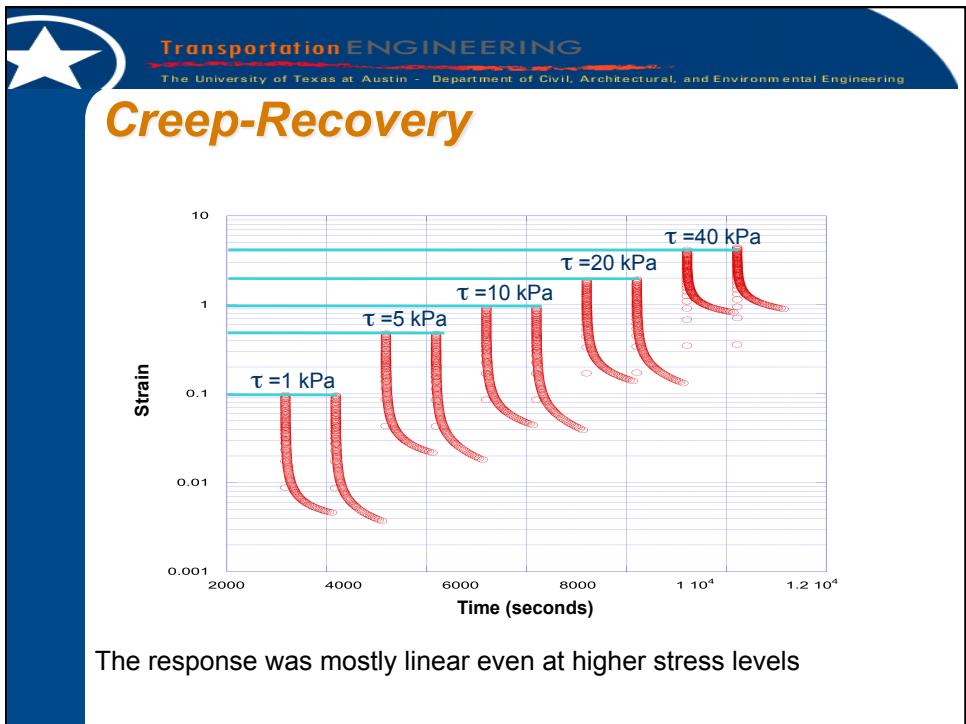
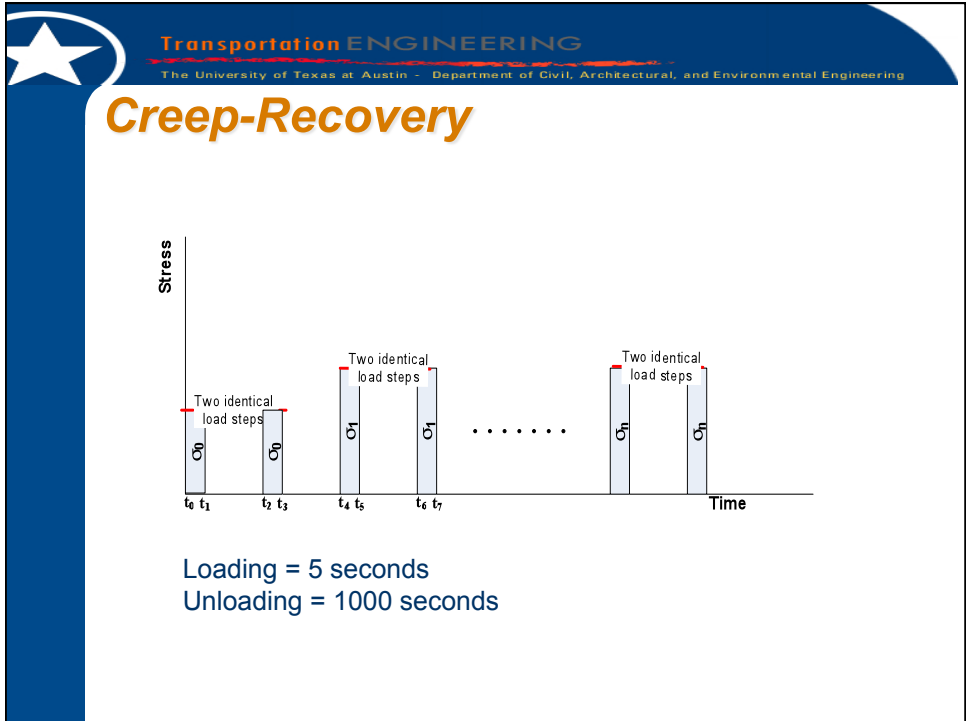
- Creep and Recovery at different stress levels:
  - Creep (loading): 5 Sec.
  - Recovery (unloading): 1000 Sec.
  - Stress level: Ranged from 100 Pa - 60 kPa
- Cyclic Loading (Stress Amplitude Sweep):
  - Freq.: 0.1 Hz
  - Amplitude Range: 1kPa – 48 kPa
- Cyclic Loading (Time Sweep at Different Stress Amplitude):
  - Freq.: 0.1 Hz
  - Amplitude Range: 1kPa – 48 kPa

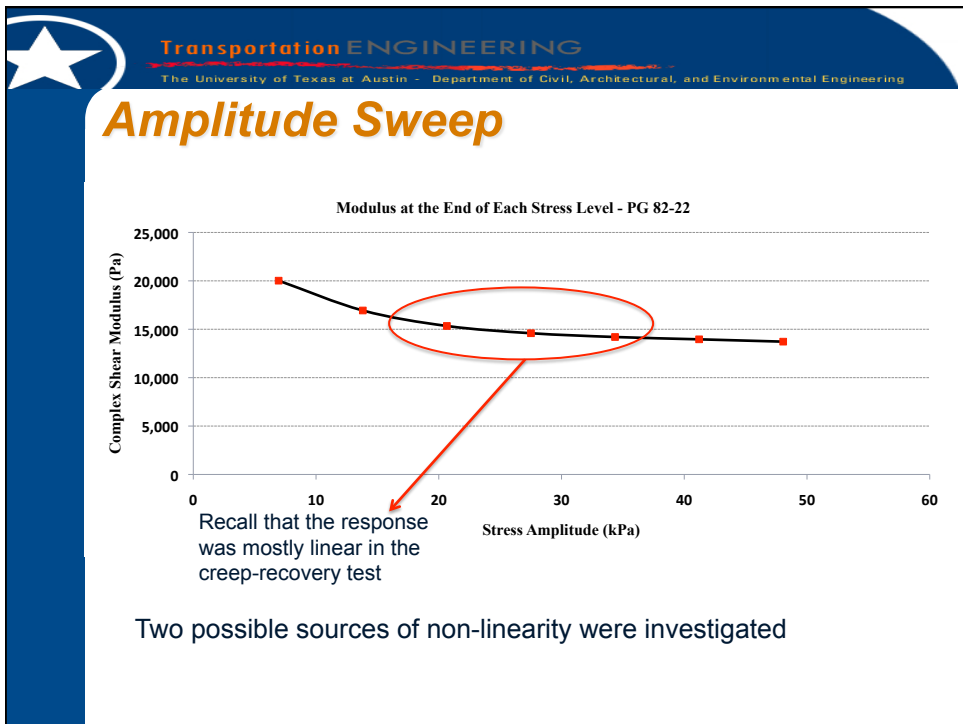
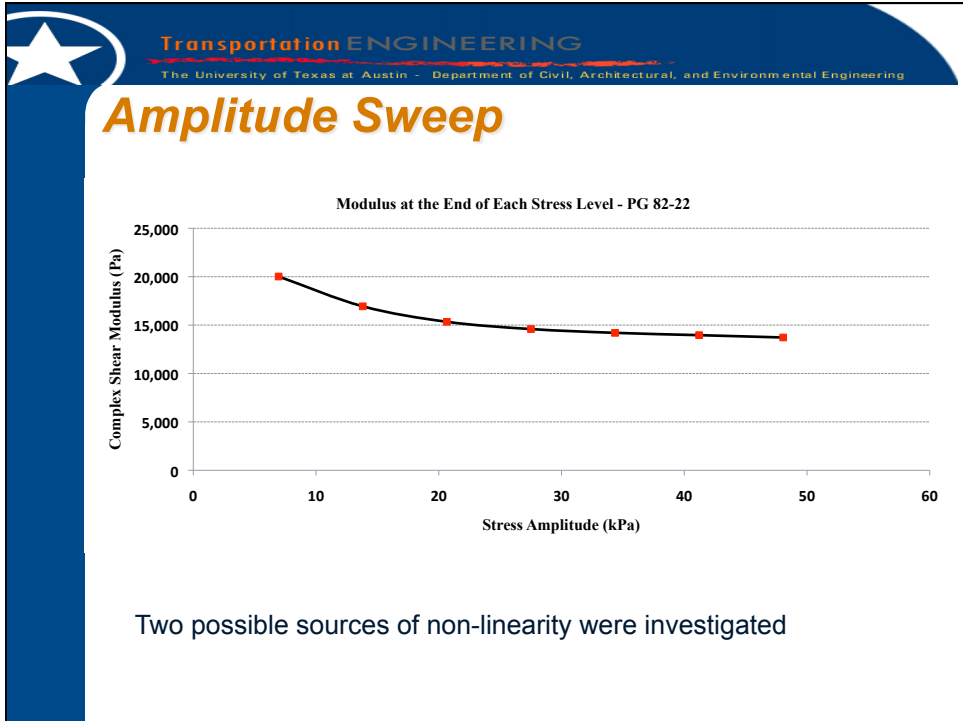
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## Test

### Selecting the stress levels

- Several studies have investigated local stresses within the mixture
- Binder can experience stresses that are approximately 80 to 100 times the far field stresses
- The applied stresses therefore reflect localized stresses when the applied far-field stresses are of the order of 1.5 kPa (30 psi)







## Analysis

Two possible sources of non-linearity were investigated

1. Inherent material non-linearity – Modulus is a function of stress (e.g.  $E = f_n(\sigma)$  or  $G = f_n(\tau)$ )
2. Interaction non-linearity – Modulus changes due to interaction of shear and normal stresses



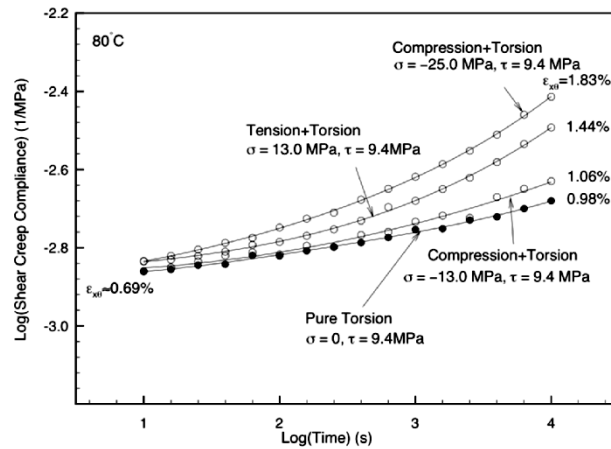
Why interaction non-linearity in a shear test?

An important attribute from torsion shear tests is the normal stress developed in the specimen during the test



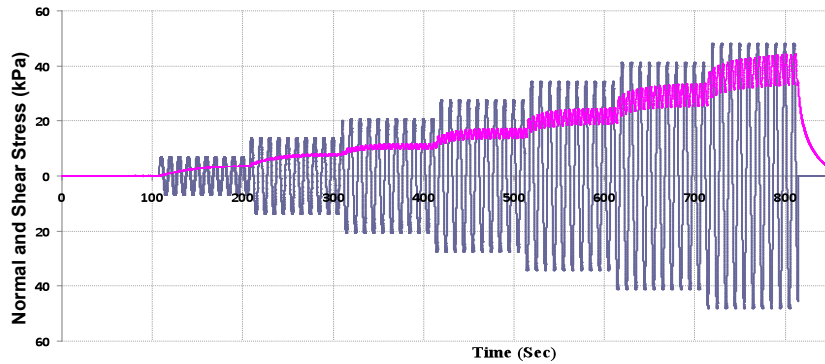
## Analysis

Interaction non-linearity



Ref: Knauss et al.

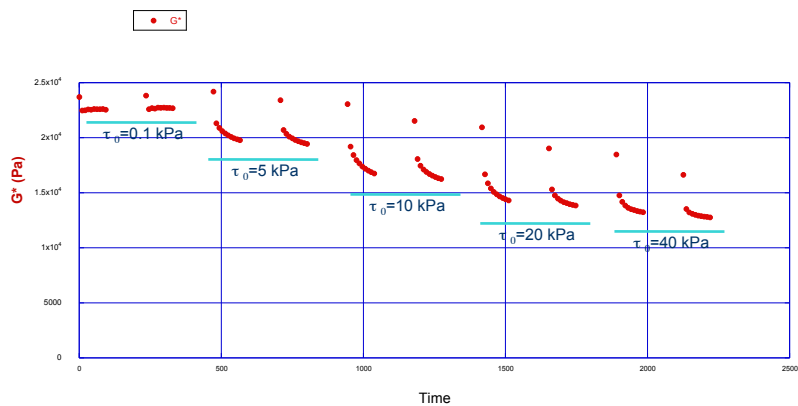
## Analysis



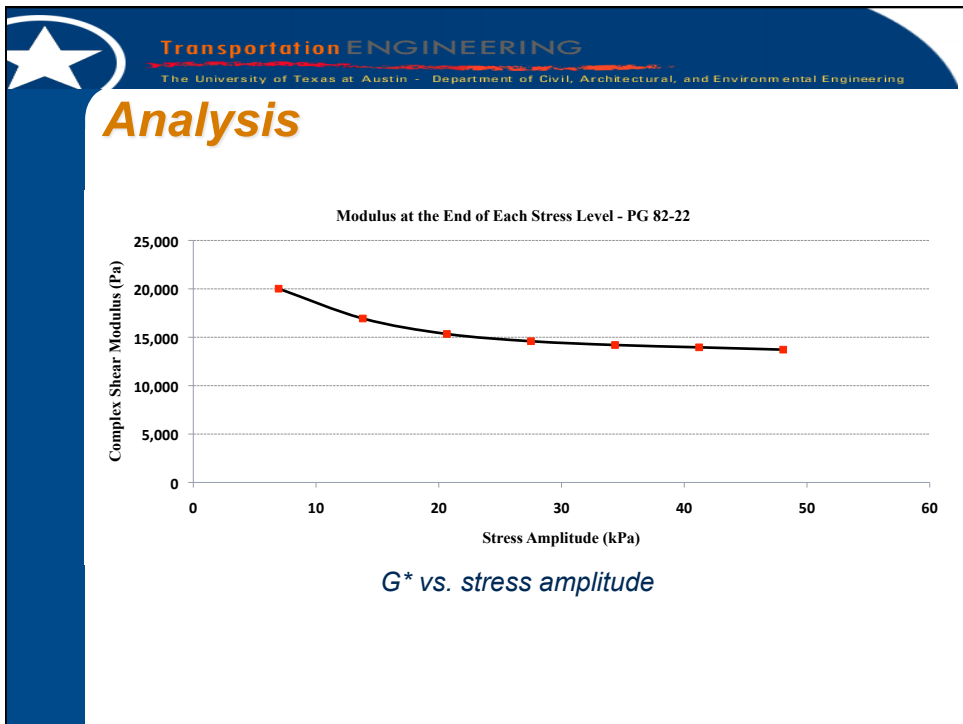
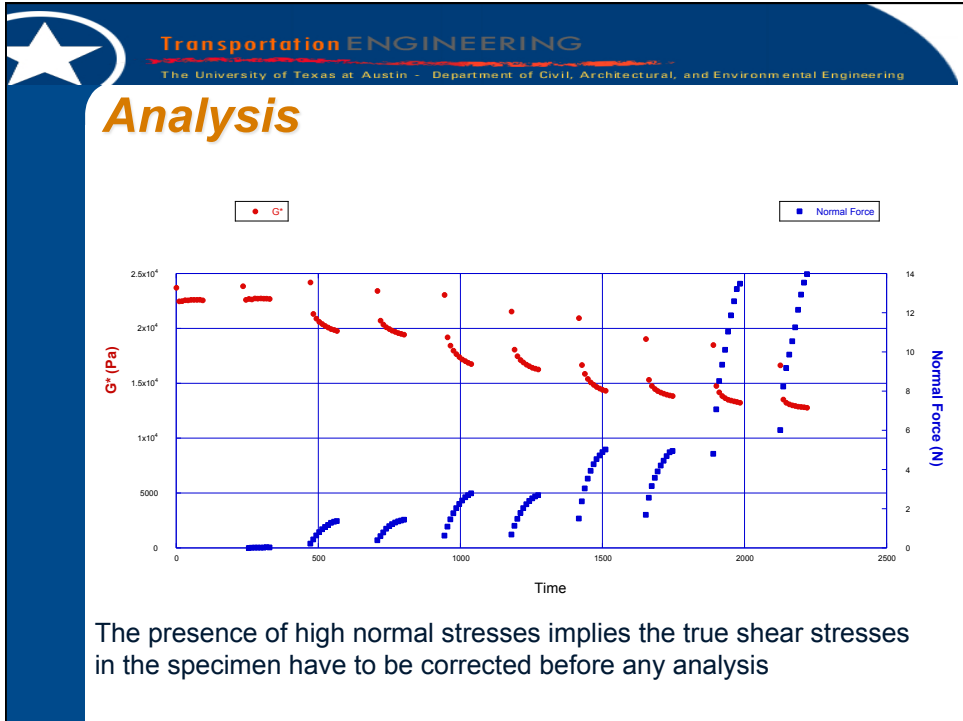
*Normal force developed in a typical amplitude sweep test*

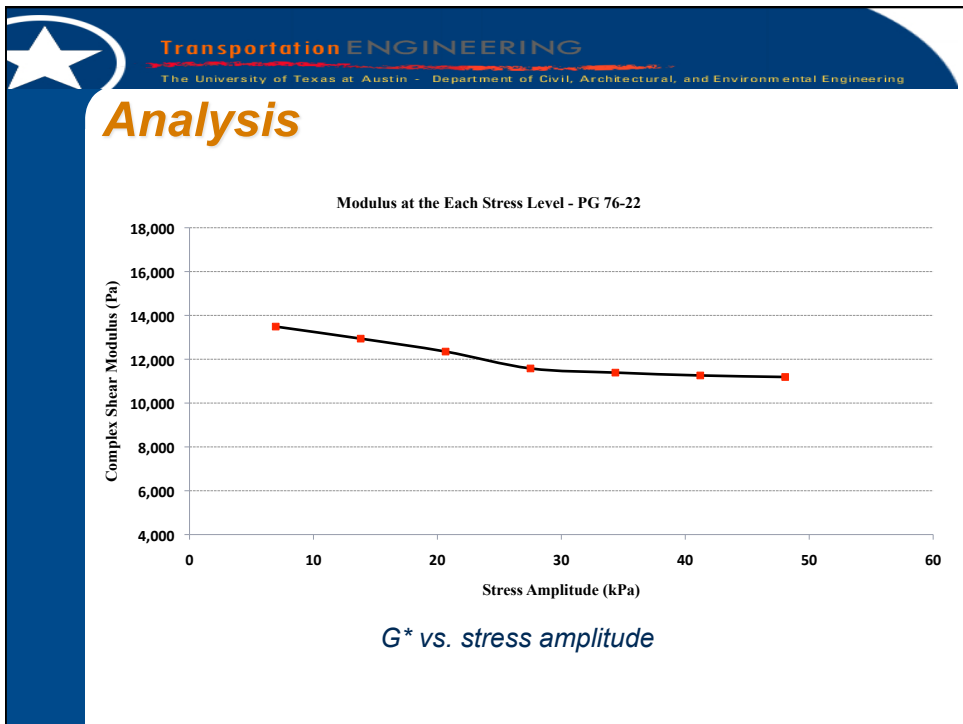
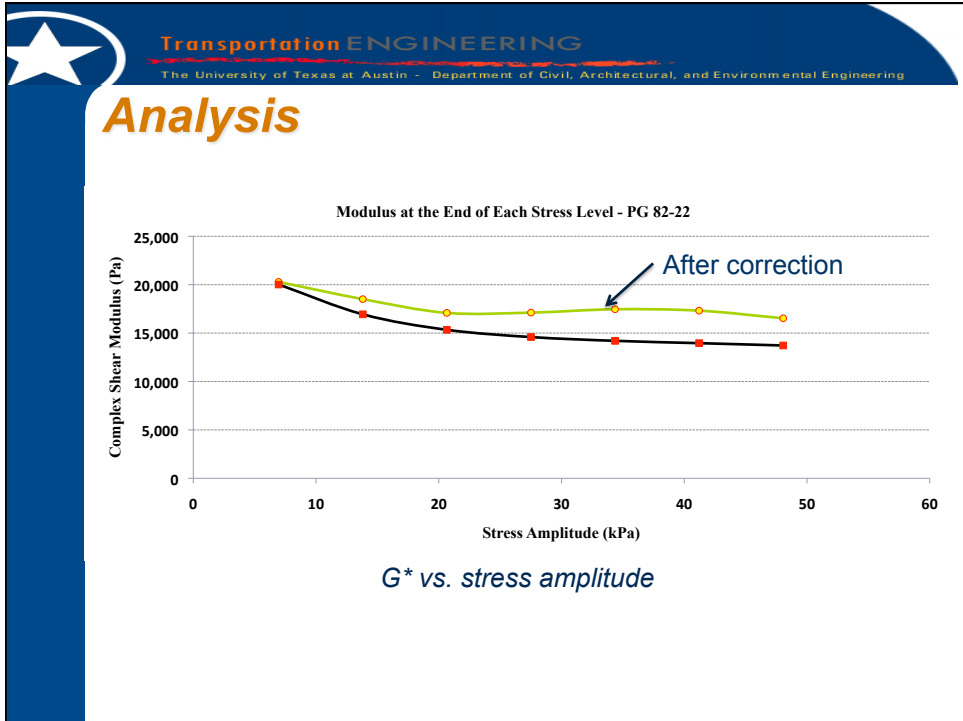
The normal force is due to the constrained geometry and the tendency of the material to expand due to (i) high strains and (ii) inherent tendency to dilate

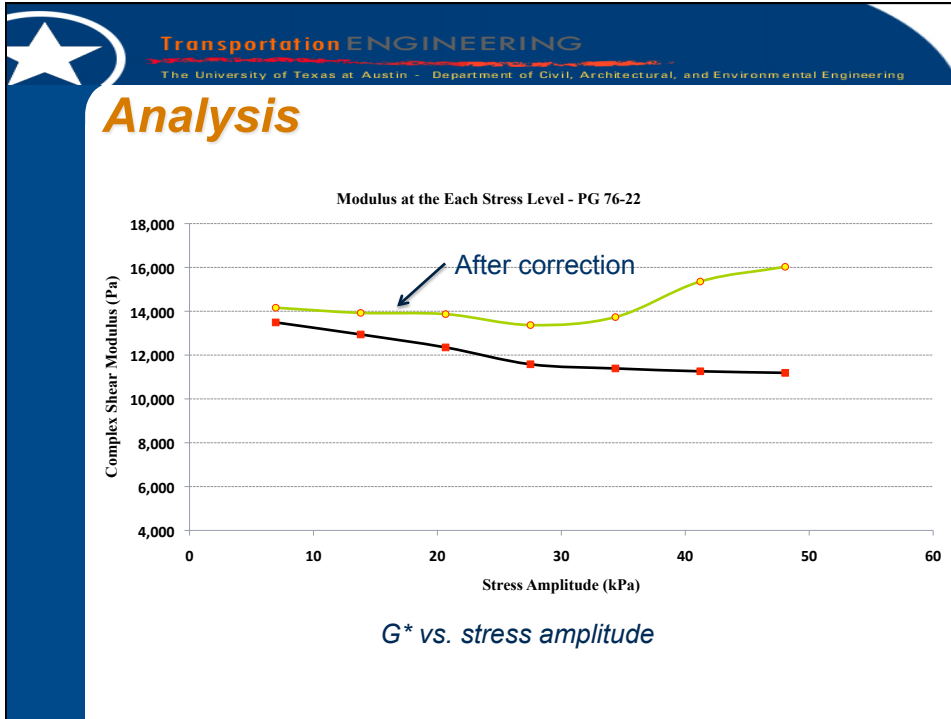
## Analysis











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## Constitutive equation

We need to incorporate the following:

1. The dilatation or first normal stress when the matrix is subjected to shear stresses
 

There are models available for this, e.g. Rivlin's model and its variations that describe first normal stress as a function of shear strain rate and shear stress
2. The non-linearity accounting for interaction between the normal and shear stresses



### Constitutive equation

Schapery's non-linear model is well suited for this case

$$\epsilon(t, \sigma) = D_0 \sigma(t) H(t) + g_1 \int_{0^-}^t \tilde{D}(t - \tau) \left\{ \frac{d[\sigma(\tau) H(\tau)]}{d\tau} \right\} d\tau$$



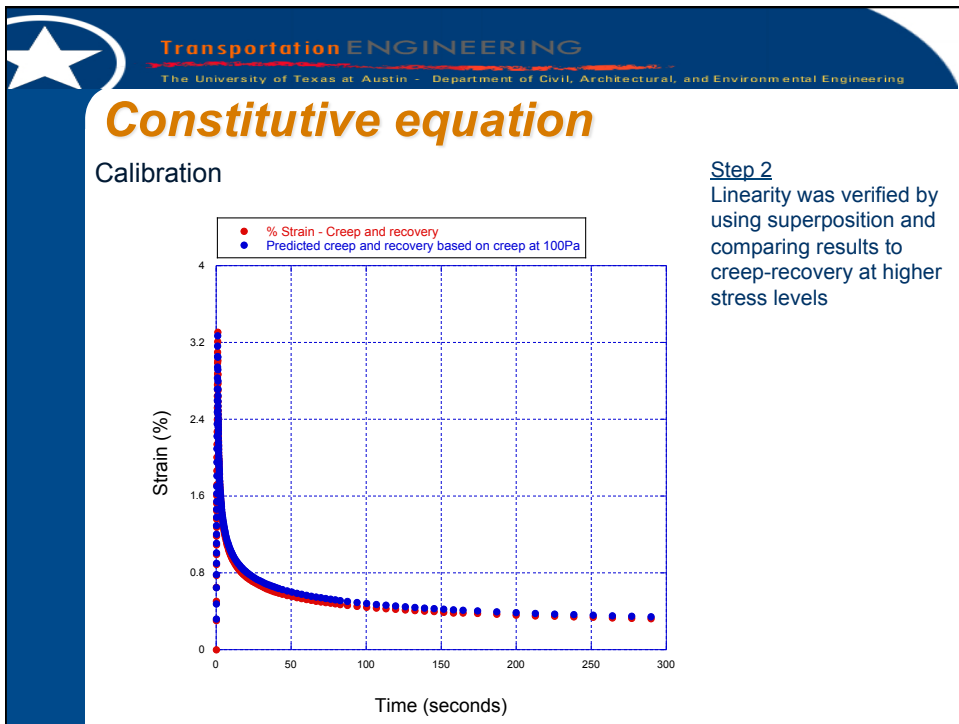
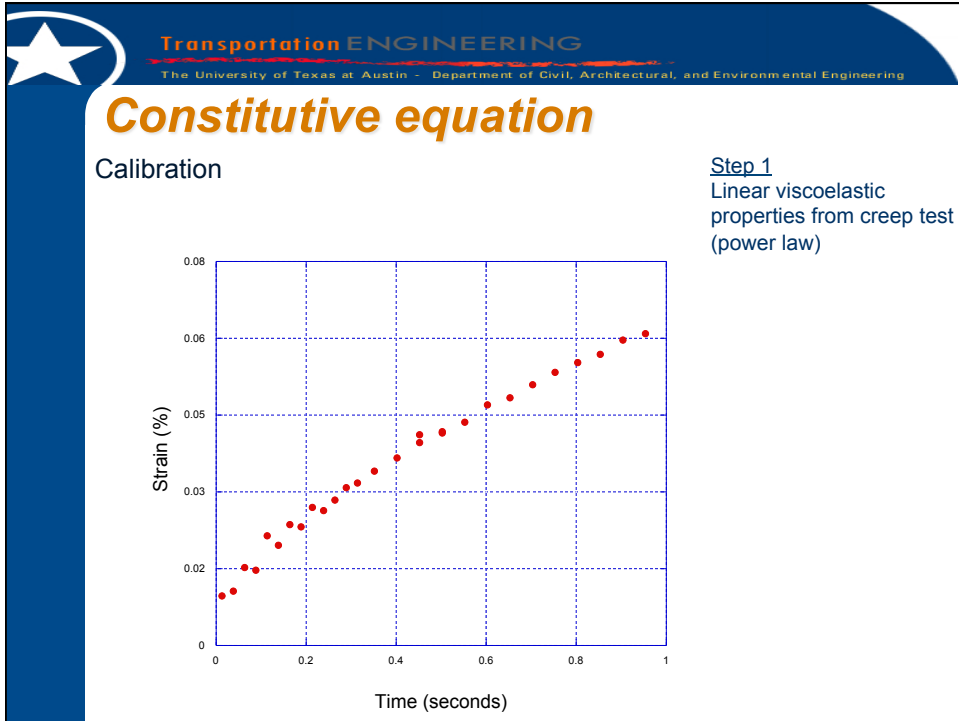
### Constitutive equation

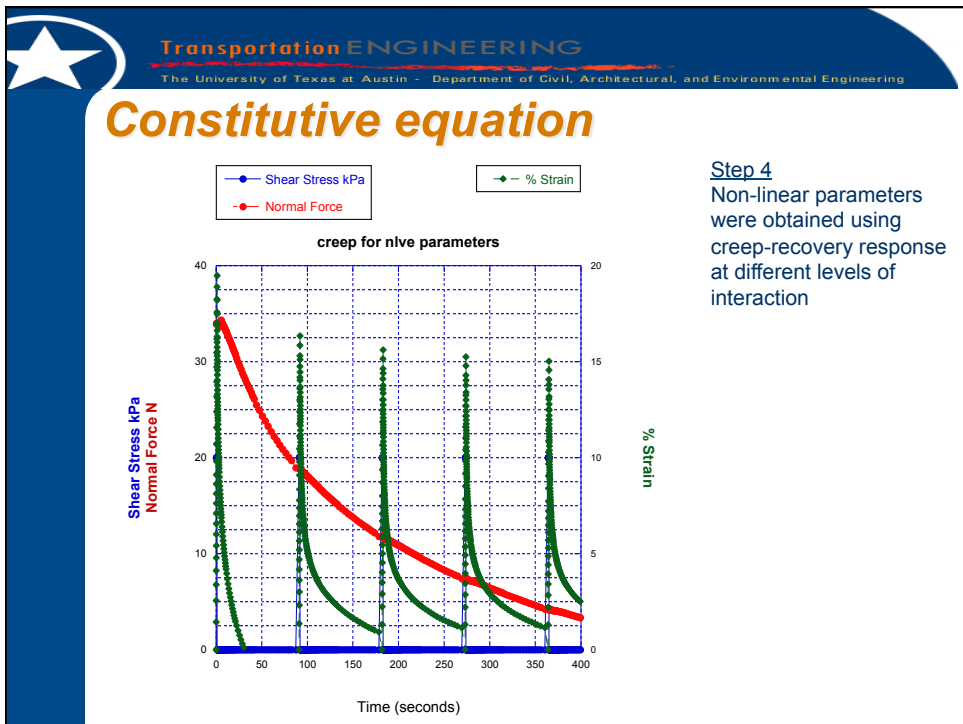
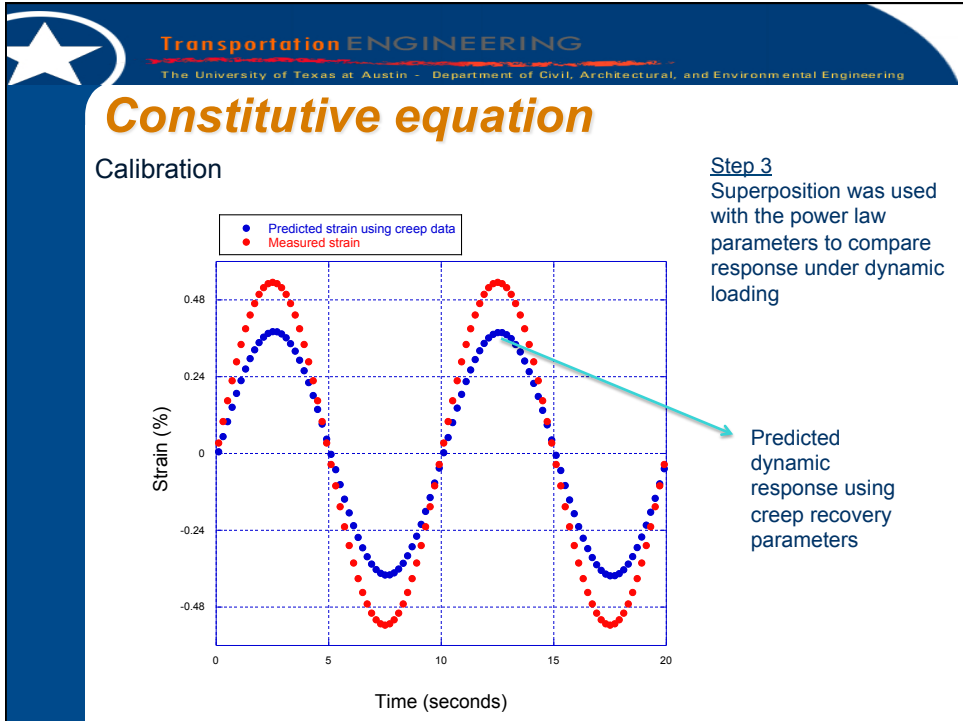
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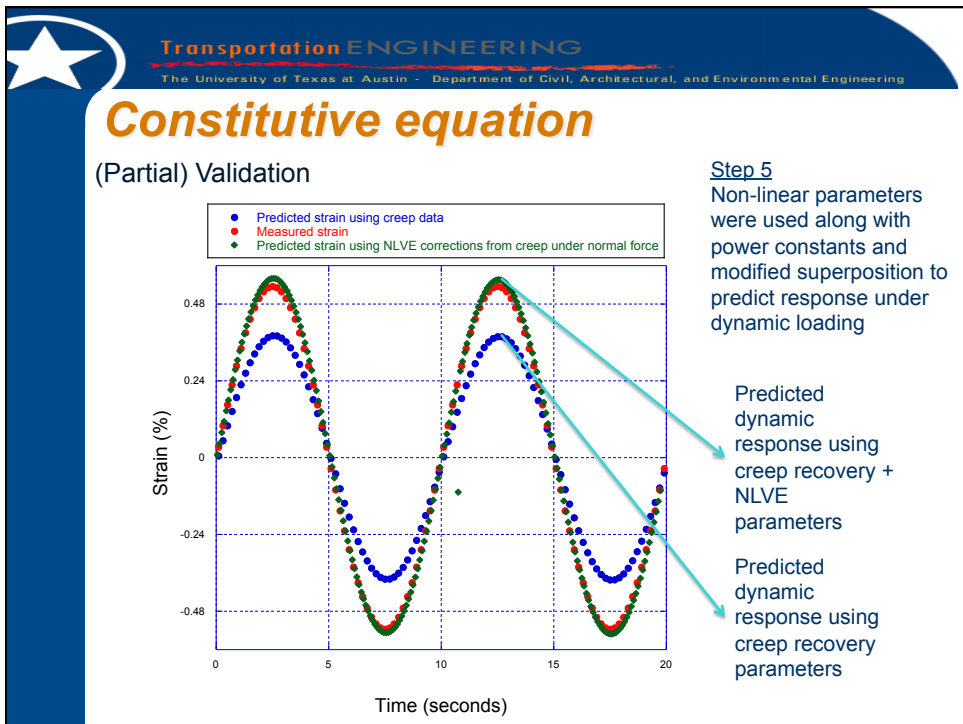
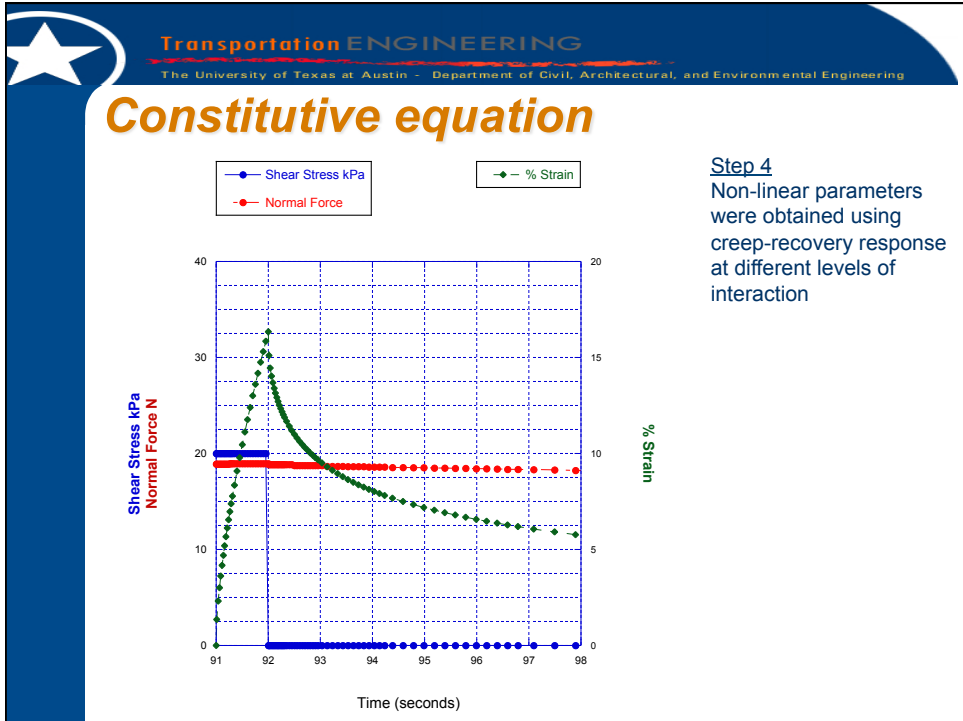
$$\epsilon(t, \sigma) = \underline{g_0} D_0 \sigma(t) H(t) + \underline{g_1} \int_{0^-}^t \underline{\tilde{D}(\psi - \psi')} \left\{ \frac{d[\underline{g_2} \cdot \sigma(\tau) H(\tau)]}{d\tau} \right\} d\tau$$

$g_i$ 's are material parameters that are dependent on the **octahedral shear stress**

Pure dependence on octahedral shear stress and path independence is currently being verified









## Conclusions

Constrained geometry in torsion shear testing can result in very high **normal stresses due to high strain and dilatation**

Dilatation is well recognized in asphalt mixtures, but it also exists in asphalt binders (as well as mastic and mortars)

A combination of normal and shear stress results in **interaction nonlinearity** which may increase or decrease stiffness of the binder and give the **impression of damage** (loss in modulus) – *this may be considered while interpreting test results*

Constitutive models are available to account for dilatation and interaction non-linearity (e.g Scahpery's NLVE model) – *this may be important to improve accuracy of computational models*

*Thanks!*

